

Water wave propagation and scattering over topographical bottoms

Zhen Ye*

Department of Physics, National Central University, Chungli, Taiwan 32054

(Received 6 October 2002; published 28 March 2003)

Here we provide a derivation of the formula recently used for investigating Bragg resonance in waves on a shallow fluid over a periodically drilled bottom [M. Torres *et al.*, Phys. Rev. E **63**, 011204 (2000)]. The equation is also compared with other existing theories. As an application, the theory is extended to the case of water waves propagating over a column with an arbitrary array of cylindrical steps. For a regular array, the formulation for computing band structures is also presented.

DOI: 10.1103/PhysRevE.67.036623

PACS number(s): 47.10.+g, 47.11.+j

I. INTRODUCTION

Propagation of water waves in water with topographical bottoms has been and continues to be a subject of much research. From the practical side, it is essential for many important ocean engineering problems such as designing underwater structures to reduce the impact of water waves on banks or floating subjects. From the research aspect, it may also hold an important value. Since water waves are a macroscopic phenomenon, they could be monitored and recorded in a laboratory scale. In this way, many significant phenomena of microscopic scales may be demonstrated with water waves. This would be particularly useful in facilitating and understanding abstract concepts pertinent to waves.

Indeed, recent experiment [1] used water waves to illustrate the phenomenon of Bloch wave, which has been previously studied in solid states. This experiment made it possible that the abstract concept be presented in an unprecedentedly clear manner. The experimental results have also been matched by a theoretical analysis in Ref. [2]. However, we found that the theory presented in Ref. [2] was not derived. Rather, it was simply given by drawing an analogy with the case of acoustic scattering through structures. There is an obvious need for a rigorous derivation of the theory. This is one of the purposes of the present paper, thereby providing a support of the formulas used in Ref. [2].

There have been many approaches for investigating propagation of water waves over various bottom topographies. A great amount of papers and monographs has been published (e.g., Refs. [3–14]). A comprehensive reference on the topic can be found in two excellent textbooks [15,16]. In this paper, we would like to derive from first principles a simple but coherent formulation for the problem. It will be shown that this formula is exactly the same as that used in Ref. [2] to explain the experimental findings in Ref. [1]. It will also be shown that this approximate approach compares favorably with existing approximations when applied to the cases considered previously. The advantage of the present approach is obvious: it is simple, accommodating, systematic, and can be easily numerically programmed. In particular, we explicitly show that it, respectively, recovers three previous results for shallow water, deep water, and scattering

by rigid cylinders standing in water. We will first give a theory for general bottom topographies. Then we will extend this to study the case of water wave propagation and scattering in a column with many cylindrical steps.

II. GENERAL THEORY

Consider a water column with an arbitrary bottom topography. We set up the coordinates as follows. Let the z axis be vertical and directed upward. The x - y plane rests at the water surface when it is calm. The depth of the bottom, which describes the bottom topography, is denoted by $h(x,y)$, and the vertical displacement of the water surface is $\eta(x,y,t)$. Now we derive the governing equations for the water waves over the bottom topography described by $h(x,y)$.

Consider a vertical column with a base differential element $dxdy$ at (x,y) . The change rate of the volume of the column is

$$\frac{\partial}{\partial t} \eta(x,y,t) dxdy.$$

By conservation of mass, this would be equal to the net volume flux from all the horizontal directions, i.e.,

$$\frac{\partial}{\partial t} \eta(x,y,t) dxdy = -\nabla_{\perp} \cdot \left[\int_{-h}^{\eta} dz \vec{v}_{\perp}(x,y,z,t) \right] dxdy,$$

where $\nabla_{\perp} = (\partial_x, \partial_y)$, and “ \perp ” denotes the horizontal directions. This gives us the first equation

$$\frac{\partial}{\partial t} \eta(x,y,t) = -\nabla_{\perp} \cdot \left[\int_{-h}^{\eta} dz \vec{v}_{\perp}(x,y,z,t) \right]. \quad (1)$$

The second equation is obtained from Newton’s second law. From the Euler equation for incompressible ideal flows,

$$\partial_t \vec{v} + (\vec{v} \cdot \nabla) \vec{v} = -\frac{1}{\rho} \nabla p - \rho g \hat{z},$$

which is valid at $z=0$, with g being the gravity acceleration, and

$$p = \rho g(\eta - z), \quad (2)$$

we obtain

*Electronic address: zhen@phy.ncu.edu.tw

$$\frac{\partial}{\partial t} \vec{v}_\perp(x, y, 0, t) + [(\vec{v} \cdot \nabla) \vec{v}]_{\perp, z=0} = -g \nabla_\perp \eta. \quad (3)$$

Note that when the liquid surface tension is included, the following term should be added to Eq. (2),

$$\sigma \nabla_\perp^2 \eta, \quad (4)$$

in which σ is the surface tension coefficient. In this paper, for simplicity we ignore this effect.

Another equation is from the boundary condition at $z = h$, which states

$$\vec{v} \cdot \hat{n}|_{z=h(x, y)} = 0, \quad (5)$$

where \hat{n} is a normal to the bottom. For an incompressible fluid, we also have the following Laplace equation:

$$\nabla \cdot \vec{v}(x, y, z, t) = 0, \quad (6)$$

in the water column. Equations (1), (3), (5), and (6) are the four fundamental equations for water waves.

A. Linearization

For small amplitude waves, i.e., $\eta \ll h$, we can ignore the nonlinear terms in Eqs. (1) and (3). Such a linearization leads to the following two equations:

$$\frac{\partial}{\partial t} \eta(x, y, t) = -\nabla_\perp \cdot \left[\int_{-h(x, y)}^0 dz \vec{v}_\perp(x, y, z, t) \right] \quad (7)$$

and

$$\frac{\partial}{\partial t} \vec{v}_\perp(x, y, 0, t) + g \nabla_\perp \eta(x, y, t) = 0. \quad (8)$$

These two equations together with Eqs. (5) and (6) determine the scattering of water waves with a bottom topography.

B. Propagation approximation

Here we provide an approximate solution to Eqs. (7), (8), (5), and (6). The procedure is as follows. When the variation of the bottom topography is smaller than the wavelength (to be determined self-consistently), we can first ignore terms involving $\nabla_\perp h$, and solve for the velocity field. For the incompressible fluid, the velocity field can be represented by a scalar field, i.e.,

$$\vec{v}(x, y, z, t) = \nabla \Phi(x, y, z, t).$$

We write all dynamical variables with a time dependence $e^{-i\omega t}$ (this time fact is dropped afterwards for convenience). This procedure leads to the following equations for Φ :

$$\nabla^2 \Phi(x, y, z) = 0 \quad (9)$$

with

$$\omega^2 \Phi(\vec{r}, 0) + g \frac{\partial}{\partial z} \Phi(\vec{r}, 0) = 0 \quad [\vec{r} = (x, y)]. \quad (10)$$

The first approximation is made at the bottom ($z = -h$). The boundary condition at the bottom reads

$$\frac{\partial}{\partial n} \Phi(\vec{r}, -h) = \frac{\partial}{\partial z} \Phi(\vec{r}, -h) + \nabla_\perp \cdot \nabla_\perp \Phi(\vec{r}, -h) = 0. \quad (11)$$

We approximate that \hat{n} is in the z direction by neglecting the second term in the above equation. This is valid as long as $\nabla_\perp h \ll kh$. Thus the boundary condition gives

$$\frac{\partial}{\partial z} \Phi(\vec{r}, -h) = 0. \quad (12)$$

Note that this condition is exact in the case of step-wise topographical bottoms, to be discussed later. Equations (9), (10), and (12) lead to the solution for Φ ,

$$\begin{aligned} \Phi(x, y, z) = & \phi(x, y) \cosh[k(z+h)] \\ & + \sum_n \phi_n(x, y) \cos[k_n(z+h)], \end{aligned} \quad (13)$$

where k satisfies

$$\omega^2 = gk(x, y) \tanh[k(x, y)h(x, y)], \quad (14)$$

and k_n satisfies

$$\omega^2 = gk_n(x, y) \tan[k_n(x, y)h(x, y)]. \quad (15)$$

Here ϕ and ϕ_n are determined by

$$(\nabla_\perp^2 + k^2) \phi = 0 \quad (16)$$

and

$$(\nabla_\perp^2 - k_n^2) \phi_n = 0. \quad (17)$$

Equation (17) leads to evanescent wave solutions.

The second approximation is to ignore the summation terms in Eq. (13). Such an approximation is based upon the following consideration. The summation terms represent the correction of evanescent waves caused by irregularities such as sudden changes of depth. As these waves are spatially confined, it is reasonable to expect that such a correction will not affect the overall wave propagation, and the general features of the wave propagation. Indeed, when we apply the later approximate solution to the extreme case of propagation of water waves over an infinite step, we find that our results agree reasonably well with that from two other approximate approaches [6,14]. For example, the difference in the reflection results is uniformly less than a few percent for a wide range of frequencies. The largest discrepancy can happen for the transmission results, but the difference is still less than 15% (Fig. 1). Furthermore, we find that the derived result is in agreement with that of Kirby for the case of waves over a flat bed with small ripples [18], i.e., when $k\delta \ll 1$ with δ being the magnitude of the ripples. As a matter of fact, in this

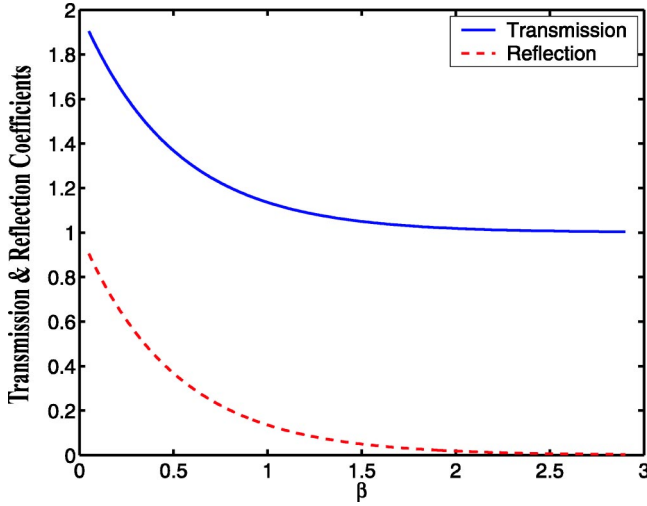


FIG. 1. The nondimensional transmission and reflection coefficients versus $\beta=kh$ for an infinite step, obtained from Eq. (22). While the result for the reflection agrees very well with that in Refs. [6,14], there is some discrepancy in the transmission results within the range of kh between 0.4 to 1.2; the largest discrepancy of about 15% occurs around $kh=0.8$ for the transmission. The legends are adopted from Ref. [6].

case, it can be shown that after a mathematical manipulation [17], Eq. (2.11) in Ref. [18] becomes essentially the same as the following Eq. (21).

Under the above approximations, we have

$$\Phi(x,y,z) \approx \phi(x,y) \cosh[k(z+h)] \quad (18)$$

and

$$v_{\perp}(x,y,z) \approx \cosh[k(z+h)] \nabla_{\perp} \phi. \quad (19)$$

Now taking Eqs. (18) and (19) into Eqs. (7) and (8), we get

$$\nabla_{\perp} \left(\frac{\tanh(kh)}{k} \nabla_{\perp} \eta \right) + \frac{\omega^2}{g} \eta = 0. \quad (20)$$

For convenience, hereafter we write ∇_{\perp} as ∇ when it acts on the surface wave field η . That is,

$$\nabla \left(\frac{\tanh(kh)}{k} \nabla \eta \right) + \frac{\omega^2}{g} \eta = 0 \quad (21)$$

or

$$\nabla \left(\frac{1}{k^2} \nabla \eta \right) + \eta = 0, \quad (22)$$

where k satisfies

$$\omega^2 = gk(\vec{r}) \tanh[k(\vec{r})h(\vec{r})]. \quad (23)$$

From this equation, we can have the conditions linking domains with different depths as follows: both η and $\tanh(kh)/k \eta = \omega^2/gk^2 \eta$ are continuous across the boundary.

Equation (22) is the same as that used in Ref. [2], and is similar to what is known as the mild-slope approximation [15]:

$$\frac{1}{c} \nabla \left(\frac{c}{k^2} \nabla \eta \right) + \eta = 0, \quad (24)$$

where c is given by

$$c = \frac{1}{2} \left(1 + \frac{2kh}{\sinh(2kh)} \right). \quad (25)$$

Equation (24) was derived by a number of authors under the situation that $\nabla h \ll kh$. In fact, under this condition it can be shown that Eq. (22) is consistent with Eq. (24).

Note that when the surface tension is added, Eq. (21) becomes

$$\nabla \cdot \left[\frac{\tanh(kh)}{k} \nabla \left(\eta - \frac{\sigma}{g\rho} \nabla^2 \eta \right) \right] + \frac{\omega^2}{g} \eta = 0, \quad (26)$$

with Eq. (23) becoming

$$\omega^2 = \left(gk + \frac{\sigma}{\rho} k^3 \right) \tanh(kh). \quad (27)$$

This is the final equation to account for water wave propagation over topographical bottom.

C. The situation of shallow water or low frequencies

In the case of shallow water, i.e., $kh \ll 1$, we obtain from Eq. (21),

$$\nabla \cdot (h \nabla \eta) + \frac{\omega^2}{g} \eta = 0. \quad (28)$$

This is the fundamental equation governing the small amplitude waves in shallow water, first derived by Lamb [3].

D. The situation of deep water or high frequencies

For the deep water case, $hk \gg 1$, we have

$$k = \frac{\omega^2}{g} \quad (29)$$

and

$$\nabla^2 \eta + \frac{\omega^4}{g^2} \eta = 0. \quad (30)$$

In the deep water, the dispersion relation is not affected by the bottom topography.

E. Scattering by infinite rigid cylinders

Equations (21) or (22) are also applicable to another class of situation which has been widely studied in the literature. That is, the scattering of water waves by infinite rigid cylinders situated in a uniform water column. When applying Eq.

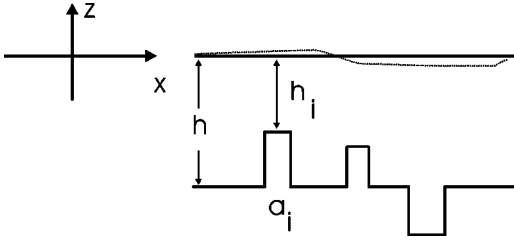


FIG. 2. Conceptual layout (side view of the three-dimensional coordinates): There are N cylindrical steps located in a water column with depth h . The depths of the steps are denoted by h_i ($i = 1, 2, \dots, N$) measured from the upper surface of the water column, and the radii of the steps are denoted by a_i . The coordinates are set up as shown. The steps are located at \vec{r}_i . The y axis lies perpendicularly to the page.

(21) or (22) to this case, we find that these two equations are actually exact. In the medium, the wave equation is

$$(\nabla + k^2)\eta = 0 \quad (31)$$

with the boundary condition at the i th cylinder,

$$\hat{n}_i \cdot \nabla \eta|_i = 0, \quad (32)$$

obtained as we set the depths of the cylinders equal zero; \hat{n}_i is a normal to the interface. In fact, in this case, the problem becomes equivalent to that of acoustic scattering by rigid cylinders, and all the previous acoustic results will follow [19–22], such as the interesting phenomenon of deaf bands.

III. WATER WAVES IN A WATER COLUMN WITH CYLINDRICAL STEPS

The problem we are now going to consider is illustrated by Fig. 2. We consider a water column with a uniform depth h . There are N cylindrical steps (or holes when $h_i > h$) located in the water. The depths of the steps are measured from the water surface and are denoted by h_i and the radii are a_i . In the realm of the linear wave theory, we study the water wave propagation and scattering by these steps.

A. Band structure calculation

When all the steps are with the same $h_1 = h_2 = \dots = h_N$ and the radius a , and are located periodically on the bottom, then we can use Bloch's theorem to study the water wave propagation. Assume the steps are arranged either in the square or hexagonal lattices, with lattice constant d . Here we use the standard plane-wave approach [23,24]. By Bloch's theorem, we can express the field η in the following form:

$$\eta(x, y) = e^{i\vec{K} \cdot \vec{r}} \sum_{\vec{G}} C(\vec{G}, \vec{K}) e^{i\vec{G} \cdot \vec{r}}, \quad (33)$$

where $\vec{r} = (x, y)$, \vec{G} is the vector in the reciprocal lattice, and \vec{K} is the Bloch vector.

In the present setup, the bottom topograph is periodic, so we have the following expansion:

$$\frac{\tanh(kh)}{k} = \sum_{\vec{G}} A(\vec{G}) e^{i\vec{G} \cdot \vec{r}} \quad (34)$$

with

$$A(\vec{G}) = \left(\frac{\tanh(k_1 h_1)}{k_1} - \frac{\tanh(kh)}{k} \right) f_s + \frac{\tanh(kh)}{k}, \quad (35)$$

for

$$\vec{G} = 0;$$

and

$$A(\vec{G}) = \left(\frac{\tanh(k_1 h_1)}{k_1} - \frac{\tanh(kh)}{k} \right) F_s(\vec{G}) \quad (36)$$

for

$$\vec{G} \neq 0.$$

Here k_1 and k are determined by

$$\omega^2 = g k_1 \tanh(k_1 h_1) = g k \tanh(kh), \quad (37)$$

and f_s is the filling factor given by [24]

$$f_s = \begin{cases} \pi \left(\frac{a}{d} \right)^2, & \text{square lattice} \\ \frac{2\pi}{\sqrt{3}} \left(\frac{a}{d} \right)^2, & \text{hexagonal lattice,} \end{cases}$$

and F_s is the structure factor

$$F_s(\vec{G}) = 2f_s \frac{J_1(|\vec{G}|a)}{|\vec{G}|a}.$$

Substituting Eqs. (33) and (34) into Eq. (21), we get

$$\sum_{\vec{G}'} Q_{\vec{G}, \vec{G}'}(\vec{K}, \omega) C(\vec{G}', \vec{K}) = 0 \quad (38)$$

with

$$Q_{\vec{G}, \vec{G}'}(\vec{K}, \omega) = [(\vec{G} + \vec{K}) \cdot (\vec{G}' + \vec{K})] A(\vec{G} - \vec{G}') - \frac{\omega^2}{g} \delta_{\vec{G}, \vec{G}'},$$

Finally, the dispersion relation connecting \vec{K} and ω is determined by the secular equation

$$\det[(\vec{G} + \vec{K}) \cdot (\vec{G}' + \vec{K})] A(\vec{G} - \vec{G}') - \frac{\omega^2}{g} \delta_{\vec{G}, \vec{G}'}]_{\vec{G}, \vec{G}'} = 0. \quad (39)$$

For the shallow water, we have $\tanh(kh) \approx kh$, and thus $\tanh(kh)/k \approx h$, then by

$$h(x, y) = \sum_{\vec{G}} A(\vec{G}) e^{i\vec{G} \cdot \vec{r}}, \quad (40)$$

with

$$A(\vec{G}) = \begin{cases} (h_1 - h)f_s + h & \text{for } \vec{G} = 0, \\ (h_1 - h)F_s(\vec{G}) & \text{for } \vec{G} \neq 0. \end{cases} \quad (41)$$

B. Multiple scattering theory

The water wave propagation in the water column with cylindrical steps can also be investigated by the multiple scattering theory. Without requiring that all the steps are the same, we can develop a general formulism, following the steps of Twersky [25].

In the water column, the wave equation reads

$$(\nabla^2 + k^2)\eta = 0, \quad (42)$$

with k being given by

$$\omega^2 = gk \tanh(kh).$$

Within the range of the i th step, the wave equation is

$$(\nabla^2 + k_i^2)\eta_i = 0 \quad (43)$$

with

$$\omega^2 = gk \tanh(kh).$$

At the boundary of the step, the boundary conditions are

$$\frac{\tanh(k_i h_i)}{k_i} \hat{n} \cdot \nabla \eta_i \Big|_{\partial\Omega_i^-} = \frac{\tanh(kh)}{k} \hat{n} \cdot \nabla \eta \Big|_{\partial\Omega_i^+}, \quad (44)$$

derived from the conservation of mass, and

$$\eta_i \Big|_{\partial\Omega_i^-} = \eta \Big|_{\partial\Omega_i^+}. \quad (45)$$

Here $\partial\Omega_i$ denotes the boundary, $+$ and $-$ denote the outer and inner sides of the step, respectively, and \hat{n} is the outward normal at the boundary.

Equations (42) and (43) with the boundary conditions in Eqs. (44) and (45) completely determine the shallow water wave scattering by an ensemble of cylindrical steps located vertically in the uniform water column. By inspecting, we see that this set of equations is essentially the same as the two-dimensional acoustic scattering by an array of parallel cylinders [21,25]. We follow Ref. [21] to study the scattering of shallow water waves in the present system.

Consider a line source located at \vec{r}_s . Without the cylinder steps, the wave is governed by

$$(\nabla^2 + k^2)G(\vec{r} - \vec{r}_s) = -4\pi\delta^{(2)}(\vec{r} - \vec{r}_s), \quad (46)$$

where $H_0^{(1)}$ is the zeroth order Hankel function of the first kind. In the cylindrical coordinates, the solution is

$$G(\vec{r} - \vec{r}_s) = i\pi H_0^{(1)}(k|\vec{r} - \vec{r}_s|). \quad (47)$$

In this section, “ i ” stands for $\sqrt{-1}$.

With N cylinder steps located at \vec{r}_i ($i=1,2,\dots,N$), the scattered wave from the j th step can be written as

$$\eta_s(\vec{r}, \vec{r}_j) = \sum_{n=-\infty}^{\infty} i\pi A_n^j H_n^{(1)}(k|\vec{r} - \vec{r}_j|) e^{in\phi_{\vec{r}-\vec{r}_j}}, \quad (48)$$

where $H_n^{(1)}$ is the n th order Hankel function of the first kind. A_n^j is the coefficient to be determined, and $\phi_{\vec{r}-\vec{r}_j}$ is the azimuthal angle of the vector $\vec{r} - \vec{r}_j$ relative to the positive x axis.

The total wave incident around the i th scatterer $\eta_{in}^i(\vec{r})$ is a superposition of the direct contribution from the source $\eta_0(\vec{r}) = G(\vec{r} - \vec{r}_s)$ and the scattered waves from all other scatterers,

$$\eta_{in}^i(\vec{r}) = \eta_0(\vec{r}) + \sum_{j=1, j \neq i}^N \eta_s(\vec{r}, \vec{r}_j). \quad (49)$$

In order to separate the governing equations into modes, we can express the total incident wave in term of the modes about \vec{r}_i ,

$$\eta_{in}^i(\vec{r}) = \sum_{n=-\infty}^{\infty} B_n^i J_n(k|\vec{r} - \vec{r}_i|) e^{in\phi_{\vec{r}-\vec{r}_i}}. \quad (50)$$

The expansion is in terms of Bessel's functions of the first kind J_n to ensure that $\eta_{in}^i(\vec{r})$ does not diverge as $\vec{r} \rightarrow \vec{r}_i$. The coefficients B_n^i are related to the A_n^j in Eq. (48) through Eq. (49). A particular B_n^i represents the strength of the n th mode of the total incident wave on the i th scatterer with respect to the i th scatterer's coordinate system (i.e., around \vec{r}_i). In order to isolate this mode on the right-hand side of Eq. (49), and thus determine a particular B_n^i in terms of the set of A_n^j , we need to express $\eta_s(\vec{r}, \vec{r}_j)$, for each $j \neq i$, in terms of the modes with respect to the i th scatterer. In other words, we want $\eta_s(\vec{r}, \vec{r}_j)$ in the form

$$\eta_s(\vec{r}, \vec{r}_j) = \sum_{n=-\infty}^{\infty} C_n^{j,i} J_n(k|\vec{r} - \vec{r}_i|) e^{in\phi_{\vec{r}-\vec{r}_i}}. \quad (51)$$

This can be achieved (i.e., $C_n^{j,i}$ expressed in terms of A_n^j) through the following addition theorem [26]:

$$\begin{aligned} H_n^{(1)}(k|\vec{r} - \vec{r}_j|) e^{in\phi_{\vec{r}-\vec{r}_j}} \\ = e^{in\phi_{\vec{r}_i-\vec{r}_j}} \sum_{l=-\infty}^{\infty} H_{n-l}^{(1)}(k|\vec{r}_i - \vec{r}_j|) e^{-il\phi_{\vec{r}_i-\vec{r}_j}} J_l(k|\vec{r} - \vec{r}_i|) \\ \times e^{il\phi_{\vec{r}-\vec{r}_i}}. \end{aligned} \quad (52)$$

Taking Eq. (52) into Eq. (48), we have

$$\begin{aligned} \eta_s(\vec{r}, \vec{r}_j) = \sum_{n=-\infty}^{\infty} i\pi A_n^j e^{in\phi_{\vec{r}_i-\vec{r}_j}} \sum_{l=-\infty}^{\infty} H_{n-l}^{(1)}(k|\vec{r}_i - \vec{r}_j|) \\ \times e^{-il\phi_{\vec{r}_i-\vec{r}_j}} J_l(k|\vec{r} - \vec{r}_i|) e^{il\phi_{\vec{r}-\vec{r}_i}}. \end{aligned} \quad (53)$$

Comparing with Eq. (51), we see that

$$C_n^{j,i} = \sum_{l=-\infty}^{\infty} i\pi A_l^j H_{l-n}^{(1)}(k|\vec{r}_i - \vec{r}_j|) e^{i(l-n)\phi_{\vec{r}_i - \vec{r}_j}}. \quad (54)$$

Now we can relate B_n^i to $C_n^{j,i}$ (and thus to A_l^j) through Eq. (49). First note that through the addition theorem the source wave can be written,

$$\eta_0(\vec{r}) = i\pi H_0^{(1)}(k|\vec{r} - \vec{r}_s|) = \sum_{l=-\infty}^{\infty} S_l^j J_l(k|\vec{r} - \vec{r}_s|) e^{il\phi_{\vec{r} - \vec{r}_s}}, \quad (55)$$

where

$$S_l^j = i\pi H_{-l}^{(1)}(k|\vec{r}_i - \vec{r}_s|) e^{-il\phi_{\vec{r}_i - \vec{r}_s}}. \quad (56)$$

Matching coefficients in Eq. (49) and using Eqs. (50), (51), and (55), we have

$$B_n^i = S_n^i + \sum_{j=1, j \neq i}^N C_n^{j,i}, \quad (57)$$

or, expanding $C_n^{j,i}$,

$$B_n^i = S_n^i + \sum_{j=1, j \neq i}^N \sum_{l=-\infty}^{\infty} i\pi A_l^j H_{l-n}^{(1)}(k|\vec{r}_i - \vec{r}_j|) e^{i(l-n)\phi_{\vec{r}_i - \vec{r}_j}}. \quad (58)$$

At this stage, both the S_n^i are known, but both B_n^i and A_l^j are unknown. Boundary conditions will give another equation relating them.

The wave inside the i th scatterer can be expressed as

$$\eta_{int}^i(\vec{r}) = \sum_{n=-\infty}^{\infty} D_n^i J_n(k_i|\vec{r} - \vec{r}_i|) e^{in\phi_{\vec{r} - \vec{r}_i}}. \quad (59)$$

Taking Eqs. (48), (50), and (59) into the boundary conditions in Eqs. (44) and (45), we have

$$B_n^i J_n(ka_i) + i\pi A_n^i H_n^{(1)}(ka_i) = D_n^i J_n(k_i a_i), \quad (60)$$

$$B_n^i J_n'(ka_i) + i\pi A_n^i H_n^{(1)'}(ka_i) = \frac{\tanh(h_i k_i)}{\tanh(hk)} D_n^i J_n'(k_i a_i), \quad (61)$$

where the prime refers to the derivative. Elimination of D_n^i gives

$$B_n^i = i\pi \Gamma_n^i A_n^i, \quad (62)$$

where

$$\Gamma_n^i = \frac{H_n^{(1)}(ka_i) J_n'(k_i a_i) - \frac{\tanh(hk)}{\tanh(k_i h_i)} H_n^{(1)'}(ka_i) J_n(k_i a_i)}{\frac{\tanh(hk)}{\tanh(k_i h_i)} J_n'(ka_i) J_n(k_i a_i) - J_n(ka_i) J_n'(k_i a_i)}. \quad (63)$$

If we define

$$T_n^i = S_n^i / i\pi = H_{-n}^{(1)}(k|\vec{r}_i - \vec{r}_s|) e^{-in\phi_{\vec{r}_i - \vec{r}_s}} \quad (64)$$

and

$$G_{l,n}^{i,j} = H_{l-n}^{(1)}(k|\vec{r}_i - \vec{r}_j|) e^{i(l-n)\phi_{\vec{r}_i - \vec{r}_j}}, \quad i \neq j, \quad (65)$$

then Eq. (58) becomes

$$\Gamma_n^i A_n^i - \sum_{j=1, j \neq i}^N \sum_{l=-\infty}^{\infty} G_{l,n}^{i,j} A_l^j = T_n^i. \quad (66)$$

If the value of n is limited to some finite range, then this is a matrix equation for the coefficients A_n^i . Once solved, the total wave at any point outside all cylinder steps is

$$\eta(\vec{r}) = i\pi H_0^{(1)}(k|\vec{r} - \vec{r}_s|) + \sum_{i=1}^N \sum_{n=-\infty}^{\infty} i\pi A_n^i H_n^{(1)}(k|\vec{r} - \vec{r}_i|) e^{in\phi_{\vec{r} - \vec{r}_i}}. \quad (67)$$

We must stress that total wave expressed by Eq. (67) incorporate all orders of multiple scattering. We also emphasize that the above derivation is valid for any configuration of the cylinder steps. In other words, Eq. (67) works for situations that the steps can be placed either randomly or orderly.

For the special case of shallow water ($kh \ll 1$), we need just replace Γ_n^i in Eq. (63) by

$$\Gamma_n^i = \frac{H_n^{(1)}(ka_i) J_n'(k_i a_i) - \sqrt{\frac{h}{h_i}} H_n^{(1)'}(ka_i) J_n(k_i a_i)}{\sqrt{\frac{h}{h_i}} J_n'(ka_i) J_n(k_i a_i) - J_n(ka_i) J_n'(k_i a_i)}. \quad (68)$$

To recover the well-known case that the water wave is scattered by cylinders standing in the water, we just need to set $h_i = 0$ in the above derivations. The previous results (e.g., Ref. [27]) will be naturally recovered.

IV. SUMMARY

In summary, here we have presented a general theory for studying gravity waves over bottom topographies. The formula used previously but without derivation in Ref. [2] is derived from first principles. The results have been extended to the case of step-wise bottom structures. The model presented here is simple and may facilitate the research on many

unusual wave phenomena such as wave localization [28], which has been tested by a landmark experiment in an one-dimensional acoustic system [29]. As can be inferred by a comparison between the formulas derived here and that for acoustic waves [21,25], water waves bear a great similarity to the acoustic waves. It has been summarized by Maynard [30] that there is an analogy between acoustics and condensed matters. Therefore, it can be expected that many

wave phenomena usually occurring in condensed matters could also be demonstrated by water waves.

ACKNOWLEDGMENTS

Discussion with H.-P. Fang and X.-H. Hu at Fudan University are appreciated. The comments from X.-H. Hu are acknowledged. This work received support from the National Science Council of the Republic of China.

-
- [1] M. Torres, J.P. Adrados, and F.R. Montero de Espinosa, *Nature* (London) **398**, 114 (1999).
- [2] M. Torres *et al.*, *Phys. Rev. E* **63**, 011204 (2000).
- [3] H. Lamb, *Hydrodynamics* (Dover, New York, 1932).
- [4] M.S. Longuet-Higgins, *J. Fluid Mech.* **1**, 163 (1956).
- [5] E.F. Bartholomeuz, *Proc. Cambridge Philos. Soc.* **54**, 106 (1958).
- [6] J.W. Miles, *J. Fluid Mech.* **28**, 755 (1967).
- [7] D.H. Peregrine, *Adv. Appl. Mech.* **16**, 9 (1979).
- [8] R.E. Meyer, *Adv. Appl. Mech.* **19**, 53 (1979).
- [9] H. Kagemoto and D.K.-P. Yue, *J. Fluid Mech.* **166**, 189 (1986).
- [10] C.M. Linton and D.V. Evans, *J. Fluid Mech.* **215**, 549 (1990).
- [11] D.V. Evans and R. Porter, *J. Fluid Mech.* **399**, 331 (1997).
- [12] T. Chou, *Phys. Rev. Lett.* **79**, 4802 (1997).
- [13] C.-L. Ting, M.-C. Lin, and C.-L. Kuo, *Phys. Fluids* **12**, 1382 (2000).
- [14] J.N. Newman, *J. Fluid Mech.* **23**, 399 (1965).
- [15] C.-C. Mei, *The Applied Dynamics of Ocean Surface Waves* (World Scientific, Singapore, 1989).
- [16] M.W. Dingemans, *Water Wave Propagation Over Uneven Bottoms* (World Scientific, Singapore, 1997).
- [17] Use the identity $\nabla_{\perp} k = -\omega^2 \nabla_{\perp} h / [c c_g \sinh(2kh)]$.
- [18] J.T. Kirby, *J. Fluid Mech.* **162**, 171 (1986).
- [19] J.V. Sánchez-Pérez *et al.*, *Phys. Rev. Lett.* **80**, 5325 (1998).
- [20] W.M. Robertson and J.F. Rudy III, *J. Acoust. Soc. Am.* **104**, 694 (1998).
- [21] Y.Y. Chen and Z. Ye, *Phys. Rev. E* **64**, 036616 (2001).
- [22] Y.Y. Chen and Z. Ye, *Phys. Rev. Lett.* **87**, 184301 (2001).
- [23] M.S. Kushwaha, *Int. J. Mod. Phys. B* **10**, 977 (1996).
- [24] Y.-Y. Chen, M. Sc. thesis, National Central University (unpublished).
- [25] V. Twersky, *J. Acoust. Soc. Am.* **24**, 42 (1951).
- [26] I.S. Gradshteyn, I.M. Ryzhik, and A. Jeffrey, *Table of Integrals, Series, and Products*, 5th ed. (Academic Press, New York, 1994).
- [27] See example, Y. Liu and D.K.P. Yue, *J. Fluid Mech.* **356**, 297 (1998).
- [28] P.W. Anderson, *Phys. Rev.* **109**, 1492 (1958).
- [29] S. He and J.D. Maynard, *Phys. Rev. Lett.* **57**, 3171 (1986).
- [30] J.D. Maynard, *Rev. Mod. Phys.* **73**, 401 (2001).